

800 East 101st Terrace, Suite 200 Kansas City, MO 64131 816-701-3100 phone 816-942-3013 fax

November 6, 2015

Ms. Leah Fitzgerald VanTrust Real Estate, LLC 4900 Main Street, Suite 400 Kansas City, MO 64112

RE: Traffic Impact Study

Meadowbrook Redevelopment

Overland Park, KS

Dear Ms. Fitzgerald

In response to your request and authorization, Wilson & Company has completed a traffic impact study for the proposed development on the old Meadowbrook Country Club in Prairie Village, Kansas. The proposed development consists of a variety of land uses encompassing roughly 138 acres.

This report summarizes the results of our traffic study. This study is focused on the impact of the proposed development on the surrounding intersections in Overland Park, Kansas, during the A.M. and P.M. peak hours of a typical weekday. Included in this study are trip generation projections, volume/capacity analyses, and improvement to the street system to mitigate the impact of the proposed development.

PROPOSED DEVELOPMENT AND STUDY AREA

The proposed development is located in Prairie Village, Kansas, at the location of the old Meadowbrook Country Club. *Figure A-1* in *Appendix A* shows the location of the proposed development and its relationship with the surrounding streets. The proposed development is bounded by Nall Avenue on the west and 95th Street and 94th Terrace on the south. The remainder of the development is bounded by residences along Somerset Drive, 90th Street, and Roe Avenue on the north and east.

The proposed land use condition includes 330 units of senior housing, 280 units of apartments, 68 units of townhomes, 57 single family home units, a 50 room Inn, and 87 acres of public park. For analysis purposes, the proposed development was further broken down into two scenarios; All of the Proposed Development Without the Park (Development), and All Proposed Development Including the Park (Development Plus Park).



Alaska

Arizona California

Colorado Illinois

Kansas

Louisiana

Minnesota

Missouri Nebraska New Mexico Oklahoma Texas

Utah



Access to the proposed development is to be provided from a main entrance at the intersection of Nall Avenue and 92nd Terrace and a secondary driveway onto 94th Terrace. A copy of the site plan showing driveway locations is included on *Figure A-2*.

95th Street is an east/west road with two-lanes in each direction and left turn lanes at various intersections. The posted speed is 35 mph adjacent to the development. Nall Avenue is a north/south road with two-lanes in each direction, and left turn lanes at its intersections with 95th Street and Somerset Drive. The posted speed is 35 mph adjacent to the development. Somerset Drive and 90th Street are both east/west streets with posted speed limits of 30 and 25 mph, respectively. Roe Avenue is a north/south roadway with a posted speed limit of 35 mph.

To assess the impacts of the proposed development, several intersections were identified for study during the peak hours. The intersections are located in the immediate area of the site and include:

- Nall Avenue and 92nd Terrace
- Nall Avenue and 94th Terrace
- Rosewood and 95th Street
- Roe Avenue and 91st Street
- Nall Avenue and 91st Street and Somerset Drive
- Nall Avenue and 95th Street
- Roe Avenue and 95th Street
- Roe Avenue and 93rd Street
- Roe Avenue and 92nd Terrace
- Roe Avenue and 90th Street

Traffic counts were taken at the intersection on typical weekdays from March 31 to April 1, 2015 from 7:00 A.M. – 9:00 A.M. and 4:00 P.M. – 6:00 P.M. The existing lane configurations and peak hour traffic volumes are shown on *Figures A-3 and A-4*.

ANALYSIS

The analysis of the proposed development's impact includes calculations of vehicle trip generation, distribution of trips onto the street network, and analyses of peak hour operations. Each of these analysis techniques and their results are described below.

TRIP GENERATION

The vehicle trips generated by the proposed development were calculated using the Institute of Transportation Engineers' <u>Trip Generation</u>, 9th Edition. The estimated daily, A.M. and P.M. peak hour traffic volumes associated with this development are shown on the following page in Table 1 and the estimated traffic volumes associated with the Park in Table 2.



Table 1: Trip Generation Proposed Development								
	T. 12 D.11		A.	M. Peak	Hour	P.N	Л. Peak I	Iour
Land Use	Intensity	ity Daily		Out	Total	In	Out	Total
Luxury Apartments	280	1,820	28	113	141	112	60	172
CCRC	330	792	40	21	61	29	36	65
Single Family Dwelling Unit	57	626	12	37	50	40	23	63
Townhomes (East)	34	252	4	18	22	17	8	25
Townhomes (West)	34	252	4	18	22	17	8	25
Inn	50	302	8	15	23	13	11	24
TOTAL		4,043	96	222	318	228	146	374

Due to limited and relatively volatile data available, a more rigorous procedure was used to project the trip generation of the proposed 87 acres of Park area. As of the date of this document the final plan for the Park has not been established, but the following is the plan at this time:

- 1. The existing clubhouse will remain and will be used on an interim basis as a local meeting space. Typical meetings may be: quarterly homes association meetings, holiday parties, wedding receptions, art classes, etc. The kitchen appliances in the facility will be removed.
- 2. The swimming pool will be removed.
- 3. There are four existing tennis courts. Two of the courts will be removed, two will remain.

ITE's Land Use: 412, County Park was used to calculate the trips generated by the Park area. ITE's description of the land use is as follows:

"County parks are owned and operated by a county. The county parks surveyed vary widely as to location, type and number of facilities, including boating or swimming facilities, ball fields, soccer fields, camp sites, picnic facilities and general open space."

Based on ITE's description, it appears that the ancillary facilities of the County Park land use will generate more traffic than the meeting space and tennis courts of the Meadowbrook site. Therefore, the county park trip generation rate was used for the entire Park area and appears to be a conservative analysis. Further, since there is substantial variability in the data, for both the AM and PM peak hours a trip generation rate for an actual data point was used instead of the average rate. The data points used can be seen on the attached figures. The data points represent a park with an area slightly less than 50 acres. Based on these sources, **Table 2** shows the Park area of the Meadowbrook Redevelopment trip generation:

Table 2: Trip Generation Park Only								
	Lateration		A.M. Peak Hour		P.M. Peak Hour			
Land Use	Intensity	Daily	In	Out	Total	In	Out	Total
County Park	86.7	198	17	11	28	84	53	137
TOTAL		198	17	11	28	84	53	137

More detailed information on trip generation calculations are included in *Appendix B*.



TRIP DISTRIBUTION

The estimated peak hour trips generated by the Proposed Development were distributed onto the street system based on existing travel patterns and expected service area of the development. **Table 3** illustrated the general distribution patterns used in this study for Development Only. The Proposed Development distributions were achieved by treating the development as a node and then distributing the entering and exiting traffic based upon existing entering and exiting usage patterns. **Table 4** shows the general distribution patterns for the Park. The detailed distribution patterns through the study intersections are documented in *Appendix B*.

Table 3: Development Trip Distribution						
To/Com Direction & Books	Ente	ering	Exiting			
To/From Direction & Route	AM	PM	AM	PM		
North on Nall Avenue	20%	10%	15%	15%		
West on 91st Street	5%	5%	5%	5%		
Northeast on Somerset Drive	5%	10%	5%	10%		
West on 95th Street	15%	20%	20%	15%		
South on Nall Avenue	25%	20%	15%	20%		
South on Roe Avenue	5%	10%	15%	10%		
East on 95th Street	15%	15%	15%	20%		
North on Roe Avenue	10%	10%	10%	5%		
TOTAL	100%	100%	100%	100%		

Table 4: Park Only Dist	Entering	Exiting
To/From Direction & Route	AM & PM	AM & PM
North on Nall Avenue	18%	19%
West on 91st Street	0%	0%
Northeast on Somerset Drive	13%	13%
West on Somerset Drive	19%	18%
West on 95th Street	14%	15%
South on Nall Avenue	16%	15%
South on Roe Avenue	6%	6%
East on 90th Street	4%	4%
East on 95th Street	6%	6%
North on Roe Avenue	4%	4%
TOTAL	100%	100%



TRAFFIC OPERATION ASSESSMENT

The operating characteristics of study area intersections were analyzed using Synchro 8.0, using methodologies from the 2010 Highway Capacity Manual (HCM) [TRB Special Report 209, 2000]. Intersection turning movement counts, the number of lanes and traffic control were used to determine existing and future levels of service. Level of service (LOS) ranges from A to F and describes traffic conditions at an intersection or on a roadway. LOS A, the highest grade, indicates a condition of little or no congestion and LOS F a condition with severe congestion, unstable traffic flow, and stop-and-go conditions. Table 5 shows the Highway Capacity Manual definitions for LOS and the corresponding delay for unsignalized and signalized intersections.

Table 5: Interse	Table 5: Intersection Level of Service Delay Thresholds						
Level of Service (LOS)	Signalized	Unsignalized					
A	< 10 Seconds	< 10 Seconds					
В	< 20 Seconds	< 15 Seconds					
С	< 35 Seconds	< 25 Seconds					
D	< 55 Seconds	< 35 Seconds					
E	< 80 Seconds	< 50 Seconds					
F	≥ 80 Seconds	≥ 50 Seconds					

For intersections, LOS is based on the average delay experienced by all traffic using the intersection during the busiest (peak) 15-minute period. LOS A through D is generally considered acceptable. Each of the aforementioned scenarios was analyzed during the weekday AM and PM peak hours. Under the existing scenario, existing cycle lengths, splits, and offsets were used in each of the time periods analyzed to reflect actual traffic operations, with signals currently being coordinated and some being fully actuated. Under the build scenarios, cycle lengths, splits, and offsets were optimized to reflect a completely coordinated signal system. The results are presented in the following summaries, and supporting calculations are presented in **Appendix C**.

Level of Service (LOS) analyses were performed using the Synchro software, which uses methodologies from the 2010 Highway Capacity Manual (HCM). The LOS values reported in this document are the HCM values.

EXISTING CONDITIONS

The results of the intersection analysis for the A.M. and P.M. peak hour existing conditions are summarized in **Table 6**. The study intersections were evaluated with the existing traffic volumes, traffic controls and lane configurations shown on *Figures A-3 and A-4*.



Table 6: Existing Co	nditions				
Intersection	A.M. Pe	eak Hour	P.M. Pe	P.M. Peak Hour	
Movement	LOS1	Delay ²	LOS	Delay ²	
Nall Avenue/Somerset Drive/91st Street					
All Movements (Signalized)	С	20.9	С	27.2	
Nall Avenue/Main Access/92nd Terrace			Na const		
EB Left/Thru/Right	C	24.5	C	16.6	
NB Left/Thru	В	10.9	Α	0.3	
Nall Avenue/94th Terrace					
WB Left/Thru/Right	C	16.5	F	68.8	
SB Left/Thru	Α	1.3	В	0.6	
Nall Avenue/95th Street					
All Movements (Signalized)	D	47.5	E	77.2	
Rosewood Drive/95th Street					
All Movements (Signalized)	Α	5.9	A	5.4	
Roe Avenue/95th Street					
All Movements (Signalized)	D	37.0	D	45.3	
Roe Avenue/93rd Street					
WB Left/Thru/Right	В	14.4	В	14.7	
SB Left/Thru	Α	7.9	A	0.1	
Roe Avenue/92 nd Terrace					
WB Left/Thru/Right	В	14.8	C	16.6	
SB Left/Thru	A	0.2	A	0	
Roe Avenue/91st Street					
WB Left/Thru/Right	В	11.4	В	12.5	
SB Left/Thru	A	0	A	0.4	
Roe Avenue/90th Street					
NB Left/Thru	A	0.5	A	0.5	
EB Left/Thru/Right	C	16.1	C	21.1	
WB Left/Thru/Right	C	15.5	C	17.9	
SB Left/Thru	Α	0.3	A	0.3	

^{1 -} Level of Service

The results indicate that all study intersections currently operate at an acceptable LOS with the exception of two intersection during the PM peak hour: Nall Avenue/94th Terrace and Nall Avenue/95th Street.

At the Nall Avenue/94th Terrace intersection the WB Left/Thru/Right was analyzed to operate at LOS F. Since the traffic volumes at this intersection do not approach the levels needed to meet traffic signal warrants, our recommendation for the westbound approach is to add a separate left-turn lane. This turn lane will isolate the unacceptable LOS to only the left-turn movement. Note that traffic on 94th Terrace, headed to destinations south, does have other, less direct, options that avoid the difficult left turn at Nall Avenue/94th Terrace. Also, based on field observations of existing traffic patterns, traffic signals on Nall

^{2 -} Delay in Seconds per Vehicle



Avenue north and south of 94th Terrace, at 91st Street and 95th Street, platoon Nall Avenue traffic such that there are numerous traffic gaps that will accommodate the left-turning traffic. The Nall / 95th Street intersection operates at a LOS E with a delay of 77.2. For the purposes of this study, this delay and LOS is considered an acceptable level of service for this intersection. Our recommendation for this intersection is to change the phasing to protected/permissive, and/or shorten the cycle. It should be noted that this signal is part of a coordinated traffic signal system on 95th Street.

Appendix C contains the output files from Synchro.

EXISTING PLUS DEVELOPMENT PLUS PARK CONDITIONS

The results of the intersection analysis for the A.M. and P.M. peak hour existing plus development plus park conditions are summarized in Table 7. The study intersections were evaluated with the existing plus development plus park traffic volumes, traffic controls and lane configurations shown on *Figures A-5 and A-6*.

Table 7: Existing Plus Development Plus Park Condition						
Intersection	A.M.	Peak Hour	P.M. 1	eak Hour		
Movement	LOS ¹	Delay ²	LOS ¹	Delay ²		
Somerset Drive/Parking Lot Driveway						
NB Left/Right	В	113	С	17.2		
WB Left/Thru	Α	7.9	Α	8.6		
EB Thru/Right	Α	0	A	0		
Nall Avenue/Somerset Drive/91st Street						
All Movements (Signalized)	С	31.1	D	45.4		
Nall Avenue/Parking Lot Driveway						
WB Right	В	10.8	В	13.6		
NB Thru/Right	Α	0	Α	0		
Nall Avenue/Main Access/92nd Terrace						
NB Left	В	11	В	10		
NB Thru	A	0	Α	0.2		
EB Left/Thru/Right	D	30.5	С	23.2		
WB Left	F	60.7	F	390.9		
WB Thru/Right	В	11.3	С	15.5		
SB Left	A	9.2	В	13.3		
SB Thru	A	0.5	A	1.7		
Nall Avenue/94th Terrace	8					
NB Left/Thru/Right	A	0	A	0		
EB Left/Thru/Right	A	0	A	0		
WB Left	F	64	F	249.2		
WB Thru/Right	В	11.3	С	16.9		
SB Left	A	9.6	В	12		
SB Thru	A	1	Α	0.8		
Nall Avenue/95th Street						
All Movements (Signalized)	D	37.4	Е	73.7		



Cocom down Access (O4th Towns co				
Secondary Access/94th Terrace SB Left/Right	В	10.8	В	11.1
EB Left/Thru	A	7.5	A	7.7
WB Thru/Right	A	0	A	0
	Λ	0	- 11	-
Rosewood Drive/95th Street				
All Movements (Signalized)	A	7.7	A	5.9
Roe Avenue/95th Street				
All Movements (Signalized)	D	38.9	D	38.9
Roe Avenue/93 rd Street				
NB Thru/Right	A	0	A	0
WB Left/Right	В	14.5	В	14.8
SB Left/Tlırıı	Α	7.9	A	8.4
Roe Avenue/92nd Terrace				
NB Thru/Right	Α	0	Α	0
WB Left/Right	В	14.8	C	16.6
SB Left/Tliru	A	8	Α	8.4
Roe Avenue/91st Street				
NB Left/Thru	A	0	Α	0
WB Left/Thru/Right	В	10.5	В	12.6
SB Left/Thru	A	7.7	A	8.5
Roe Avenue/90th Street				
NB Left/Thru	A	8.4	A	8.3
EB Left/Thru/Right	C	16.2	C	21.4
WB Left/Thru/Right	C	15.9	С	19.2
SB Left/Thru	A	8	A	8.4

Results from the analysis indicate the following intersections do not operated at an acceptable level of service:

- Nall Avenue / 95th Street
- Nall Avenue / Main Access / 92nd Terrace
- Nall Avenue / 94th Terrace.

The results indicate that the Nall / 95th Street intersection will operate with less delay due to the change in signal phasing from protected only to protected/permissive.

At the Nall Avenue/Main Access/92nd Terrace intersection a single lane westbound approach was analyzed to operate at LOS F. Since the Build traffic volumes at this intersection do not approach the levels needed to meet traffic signal warrants, the recommendation for the westbound approach is to add a separate left-turn lane. This turn lane will isolate the unacceptable LOS to only the left-turn movement. Note that westbound traffic on the Main Access, headed to destinations south, does have other, less direct options that avoid the difficult left turn at Nall Avenue/Main Access/92nd Terrace. Also, based on field observations of existing traffic patterns, traffic signals on Nall Avenue north and south of Main Access/92nd Terrace, at 91st Street and 95th Street, platoon Nall Avenue traffic such that there are numerous traffic gaps that will accommodate the relatively low volume of left-turning traffic.



The Nall Avenue / 94th Terrace intersection should have a separate left turn lane to isolate the poor level of service. Like the Nall Avenue / Main Access / 92rd Terrace access, the traffic signals platoon traffic on Nall Avenue north and south of the intersection such that there are gaps to accommodate the left-turning vehicles.

The Nall Avenue/Parking Lot Driveway operates at an acceptable level of service and will be a right-in right-out only intersection due to the proximity to the Nall Avenue / Somerset Drive intersection. Table 8 shows the available queue length and calculated queues. The northbound Nall Avenue / Somerset Drive / 91st Street queue has 187' of available queue length. The PM northbound queue is expected to exceed the left-turn bay by 53' feet or approximately 2 vehicles. This queue may back up beyond the park entrance driveway to the south. The right-in right-out configuration at the driveway will address concerns of the lengthy queues.

Appendix C contains the output files from Synchro.

Table 8: 95th Build Percenti	Table 8: 95th Build Percentile Queue Lengths					
Intersection (Movement)	Bay / Link	AM Peak	PM Peak			
	Length (feet)	95 th Queue	95 th Queue			
Nall Ave / West 95 th Street Overall Intersection (Signalized)						
Eastbound Left Eastbound Thru Eastbound Thru / Right Westbound Left Westbound Thru Westbound Thru Westbound Thru / Right Northbound Left Northbound Thru Northbound Thru Northbound Thru Southbound Thru	200	85	252			
	N/A	175	333			
	N/A	151	362			
	147	155	134			
	N/A	241	153			
	N/A	216	174			
	140	63	219			
	N/A	209	598			
	N/A	136	518			
	160	85	96			
	N/A	201	160			
	N/A	209	160			
Eastbound Left Eastbound Thru / Right Westbound Left Westbound Thru / Right Westbound Thru / Right Northbound Left Northbound Thru Northbound Right Southbound Left Southbound Thru Southbound Thru	149	152	211			
	N/A	249	490			
	65	159	162			
	N/A	231	210			
	187	183	240			
	N/A	288	384			
	N/A	22	93			
	182	37	41			
	N/A	184	167			
	N/A	118	140			



Intersection (Movement)	Bay / Link Length (feet)	AM Peak 95th Queue	PM Peak 95 th Queue
Roe Avenue / West 95th Street	Length (rect)	70 Queue	yo Queuc
Overall Intersection (Signalized)			
(8			
Eastbound Left	146	54	147
Eastbound Thru	N/A	117	368
Eastbound Thru / Right	N/A	130	368
Westbound Left	116	180	91
Westbound Thru	N/A	309	155
Westbound Thru / Right	N/A	289	154
Nortlibound Left Nortlibound Thru / Right	38	37	131
Southbound Left	N/A	209	257
Southbound Thru / Right	76	94	89
	N/A	280	211
Roe Avenue / West 90th Street			
Overall Intersection (Unsignalized)			
Eastbound Left / Thru / Right	N/A	41	48
Westbound Left / Thru / Right	N/A	30	36
Northbound Left / Thru / Right	N/A	0	35
Southbound Left / Thru / Right	N/A	0	25
Roe Avenue / West 91st Street	14/11		20
Overall Intersection (Unsignalized)			
(
Westbound Left / Right	N/A	26	28
Southbound Left / Thru	N/A	0	147
Nall Avenue / West 92nd Terrace / Main Access			
Overall Intersection (Unsignalized)			
	_		_
Eastbound Left / Thru / Right	N/A	53	0
Westbound Left	N/A	0	35
Westbound Thru / Right	N/A	51	30
Northbound Left / Thru Southbound Left / Thru	N/A	0 71	26
Southbound Thru / Right	N/A	71 82	19 85
Nall Avenue / West 94th Terrace	N/A	02	00
Overall Intersection (Unsignalized)			
Overall Intersection (Onsiknanzed)	c.	1	
Westbound Left	N/A	37	48
Westbound Thru / Right	N/A	52	85
Northbound Thru / Right	N/A	19	0
Southbound Left / Thru	N/A	109	110
Southbound Thru / Right	N/A	95	74



Intersection (Movement)	Bay / Link Length (feet)	AM Peak 95th Queue	PM Peak 95th Queue
Roe Avenue / West 93 rd Street	nengui (rect)	35 Queue	oo Queue
Overall Intersection (Unsignalized)			
,			
Westbound Left / Right	N/A	43	36
Southbound Left / Thru	N/A	27	0
Rosewood / West 94th Terrace / West 95th Street			
Overall Intersection (Signalized)			
Eastbound Left	132	37	66
Eastbound Thru	N/A	37	73
Eastbound Thru / Right	N/A N/A	50	62
Westbound Left	147	66	56
Westbound Thru	N/A	0	65
Westbound Thru / Right	N/A N/A	43	83
Northbound Left	108	111	87
Northbound Thru / Right	N/A	78	58
Southbound Left	82	89	105
Southbound Thru / Right	N/A	43	104
West 94th Terrace / Secondary Access	11/11		
Overall Intersection (Unsignalized)			
(
Eastbound Left / Thru	N/A	0	38
Southbound Left / Right	N/A	54	49
Roe Avenue / West 92 nd Terrace		1	
Overall Intersection (Unsignalized)			
747 d	NT / A	40	40
Westbound Left / Right	N/A	40	40
Southbound Left / Thru	N/A	0	45
Parking Lot / Somerset Drive			
Overall Intersection (Unsignalized)			
Northwest Left / Right	N/A	0	22
Parking Lot Right-In Right-Out / Nall Avenue			
Overall Intersection (Unsignalized)			
,			
Westbound Right	N/A	0	27
Northbound Thru	N/A	81	325
Northbound Thru/Right	N/A	0	264

Geometry

The Rosewood Driveway/ 94^{th} Terrace roadway segment, from 95^{th} Street to Nall Avenue, currently has thirteen driveways. Two of these driveways are located within 50 feet of the intersection with 95^{th} Street. Modern access design would not allow the driveways to be placed this close to a signalized intersection.



Addition of a raised median on Rosewood to make the driveways right-in right-out would increase safety along the corridor.

The sight distance required for a 35 mph roadway with a decline of 6% is 278 feet and for 25 mph on level roadway it is 155' according to the KDOT Access Management Policy. Table 9 below shows the new intersections with their sight distance.

Table 9: Sight Distance							
Intersection	Speed Limit	Required Sight	Provided Sight				
		Distance	Distance				
Nall Avenue / Main Access / 92nd Terrace	35	278′	600′				
Rosewood / Secondary Access	25	155′	350′				

WALKABILITY ASSESSMENT

The purpose of the Walkability Assessment is to review the pedestrian access to and around the perimeter of the property. *Figure A-9* shows the project location and the perimeter locations within the property where sidewalk currently exists and where sidewalk will be added.

CRASH HISTORY

As part of the review of the Eastern Access/Roe Avenue connection, the crash history of the immediate area on Roe Avenue was reviewed. Table 10 shows the reported crashes during the most recent 5-years' worth of records.

Table 10: Roe Avenue Crashes								
	PDO		Injury		Fatal		Sequence of Events	
	SB	NB	SB	NB	SB	NB	1st Crash in Row	2nd Crash in Row
							Ran off Road Right, Hit	
2010	1	0	0	0	0	0	Fixed Object	
2011	0	0	0	0	0	0		
2012	0	0	0	0	0	0		
								Ran off Road Right, Hit Fixed
2013	0	1*	0	1	0	0	Hit Fixed Object	Object, Overturned
							Hit Fixed Object, Ran off	
							Road Left, Hit Fixed	
2014	0	0	1*	0	0	0	Object	

^{*}Indicates DUI

There were a total of four crashes in a five-year span with two being property damage only and two injury. Of those four crashes, two were marked DUI. The locations of three of the four crashes is



south of the intersection of Roe Avenue and W 91st street. The last crash is located at the intersection.

SUMMARY

This study documents the traffic impact of the proposed Redevelopment of the Meadowbrook Country Club on the roadway network in the vicinity of 91st Street, 95th Street, Nall Avenue and Roe Avenue in Prairie Village, Kansas. This report includes the analysis of the intersections adjacent to and surrounding the proposed development for Existing and Existing Plus Development Plus Park scenarios.

The operational analysis of existing traffic volumes shows that the existing roadway network operates within desirable levels of service with the exception of the following intersections:

- Nall Avenue/94th Terrace addition of a westbound left-turn lane will isolate the poor LOS to only the left-turn.
- Nall Avenue/95th Street addition of permissive left-turn to all approaches will maintain or improve the delay at the intersection and maintain the current LOS.

The operational analysis of existing plus proposed development traffic volumes shows that the roadway network needed to accommodate existing traffic volumes operates within desirable levels of service with the exception of the following intersection:

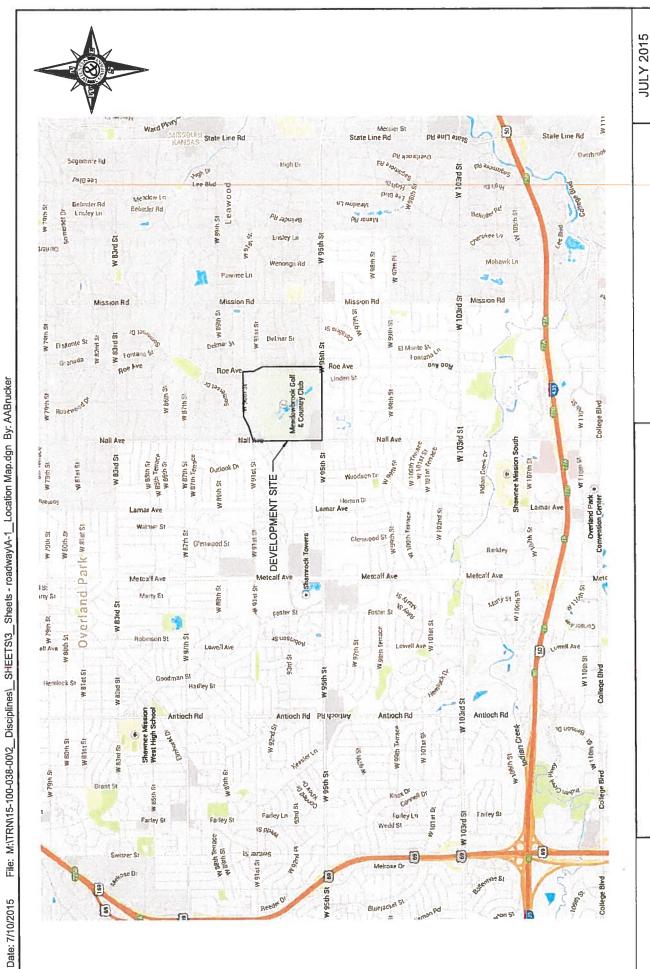
• Nall Avenue/Main Access/92nd Terrace – addition of a westbound left-turn lane will isolate the poor LOS to only the left-turn.

At the following two locations, due to access driveways located close to signalized intersections, the elimination of the driveway should be considered or a raised median constructed:

- Nall Avenue/Parking Lot
- North leg of Rosewood Drive/95th Street

At the time of the publication of this report there are two outstanding intersection questions:

- 1. The connection to 94th/Rosewood is being negotiated. The configuration shown in this report appears to be the final plan, however, negotiations are ongoing at this time.
- 2. There has been discussion of left-turn lane(s) on Nall Avenue at the Main entrance. The intersection has an acceptable LOS without the left-turn lane(s), therefore, the LOS will only improve with the addition of southbound and/or northbound left-turn lane(s).



LOCATION MAP

FIGURE A-1

NO SCALE

MEADOWBROOK DEVELOPMENT PRAIRIE VILLAGE, KANSAŞ

WILSON

File: M:\TRN\15-100-038-00\2_Disciplines_SHEETS\3_Sheets - roadway\A-2_Site Plan.dgn Date: 11/6/2015

By: aabrucker

MEADOWBROOK DEVELOPMENT PRAIRIE VILLAGE, KANSAS

JULY 2015 NO SCALE

FIGURE A-2

WILSON &COMPANY

SITE PLAN

12(15) 414(383) 16(13) **LEGEND** AM (PM) PEAK HOUR TRAFFIC VOLUME 121(99) 11(21) 5(7) 7(9) 2(12) 21(25) 20(30)-289(429)-8(13)-19(22) 24(50) 673(587) 13(12) SOMERSET DRIVE 4(12) 148(183) W 90TH STREET 206(165) W 91ST STREET 391(396) 44(61) 121(217) 20(21) 50(117)-444(715)-105(251)-96(85) - 5(3) W 91ST STREET NALL AVENUE 305(454) -1(2) -406(397) 10(9) ROE AVENUE File: M:\TRN\15-100-038-00\2_Disciplines_SHEETS\3_Sheets - roadway\A-3_Existing Peak Hour Traffic Volumes.dgn 1(1) 1050(839) 6(9) **— 22(17)** W 92ND TERRACE W 92ND TERRACE 4(1) 9(8) 297(446) 6(18) 2(7) -614(1180) -423(387) 6(5) 988(852) 66(18) - 8(8) **— 21(8)** 34(104) 5(28) W 93RD STREET W 94TH TERRACE 291(450) 6(12) 602(1076) -58(14) -76(60) 357(279) 52(38) 39(81) 691(656) 64(107) 5(32) 8(9) 25(61) 41(48) 44(44) 58(83) - 841(599) - 95(65) 581(775) 656(637) 167(157) 43(41) W 95TH STREET 46(88) 536(953) 13(22) 73(120) 585(1010) 439(774) 36(45) – 208(351) – 38(61) – 56(128) -517(870) -135(163) -61(42) 14(4) 71(41) 40(59) 70(93) 21(10) ROSEWOOD DRIVE **JULY 2015** By: aabrucker WILSON MEADOWBROOK DEVELOPMENT PRAIRIE VILLAGE, KANSAS **EXISTING PEAK HOUR** NO SCALE &COMPANY TRAFFIC COUNTS FIGURE A-3

Date: 11/6/2015

File: M:\TRN\15-100-038-00\2_Disciplines_SHEETS\3_Sheets - roadway\A-4_Existing lane configurations.dgn

Date: 11/6/2015

A-7_Developement Plus Park Peak Hour Traffic Volumes.dgn 28 Sheets - roadway\2015_10_ File: M:\TRN\15-100-038-00\2_Disciplines_SHEETS\3_

> Date: 11/6/2015 By: aahriicker

&COMBANY WILSON

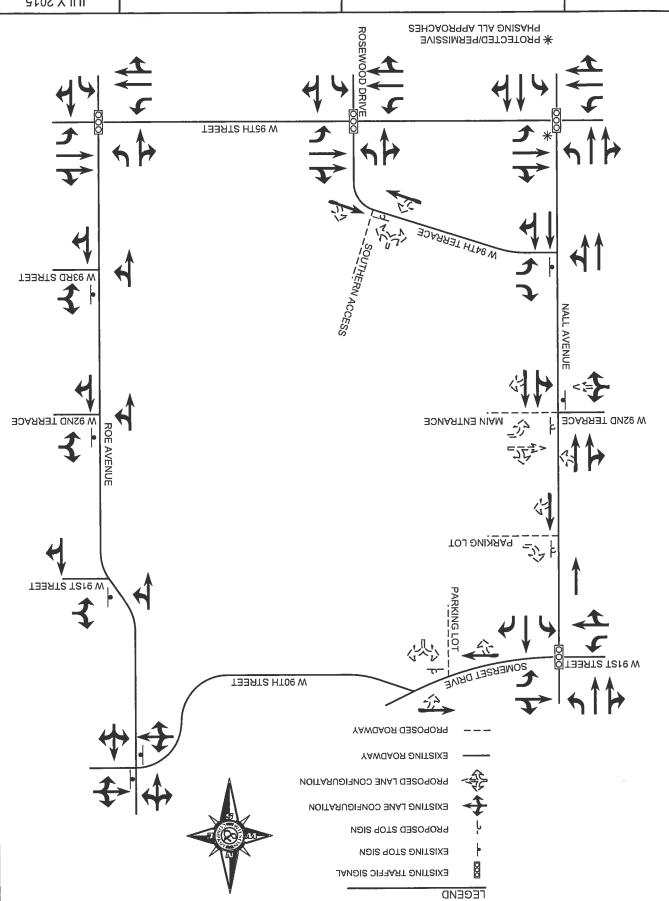
Date: 11/6/2015 By: aabnucker

EXISTING PLUS PARK DEVELOPMENT PLUS PARK

MEADOWBROOK DEVELOPMENT PRAIRIE VILLAGE, KANSAS

FIGURE A-6 NO SCALE

2015

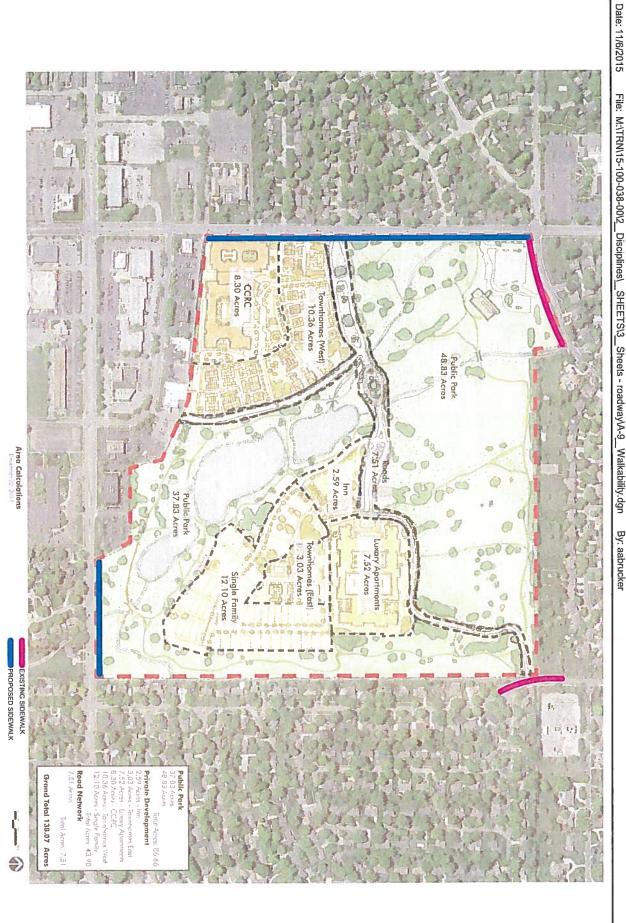


File: M:\TRN\15-100-038-00\2_Disciplines_SHEETS\3_Sheels - roadway\2015_10_28_A-8_Development Plus Park Lane Configurations.dgr

MEADOWBROOK DEVELOPMENT PRAIRIE VILLAGE, KANSAS

NO SCALE **JULY 2015**

FIGURE A-7



File: M:\TRN\15-100-038-00\2_Disciplines_SHEETS\3_Sheets - roadway\A-9_Walkability.dgn